LUTAK DOCK REHABILITATION PROJECT CITY OF HAINES, ALASKA

EDA PROJECT NO. 07-79-4967

AB ADJ AGGR ALT ALUM ANCH

ASTM

Countersunk

Reid Middleton

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IN ASSOCIATION WITH:



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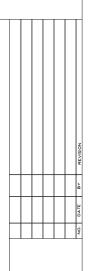


Snow Load, Slope

STRUCTURAL ABBREVIATIONS

	And	DR	Diaphragm Boundary	GA.	GA.	(N)	New	SOG	Slab-on-Grade
	At	DBL	Double	GALV	Galvanized	NFS	Non-Frost Susceptible	SPCG	Spacing
	Center Line	DEMO	Demolition	GLB	Glue-Laminated Beam	NIC	Not in Contract	SPEC	Specification
		DIAG	Diagonal	GR	Grade	NO	Number	SQ	Square
	Anchor Bolt	DICA	Drilled-in-Concrete Anchor	GWB	Gypsum Wallboard	NOM	Nominal	SSH	Short Slotted Hole
	Adjustable, Adjacent	DIM	Dimension		,,	NS	Near Side	STT	Stainless Steel
	Aggregate	DIMA	Drilled-in-Masonry Anchor	HD	Hold-down	NTS	Not to Scale	STD	Standard
	Alternate	DL	Dead Load	HDG	Hot-Dip Galvanized			STFNR	Stiffener
	Aluminum	DN	Down	HDR	Header	OA	Overall	STL	Steel
	Anchor, Anchorage	DO	Ditto	HOR	Horizontal	OC	On Center	STOR	Storage
X	Approximate	DTL	Detail	HT	Height	OD	Outside Diameter (Dim.)	STRUCT	Structural
	Architectural, Architect	DWG	Drawing	HSL	Horizontal Long Slotted Holes	OPNG	Opening	SYM	Symmetrical
	American Society for	DIIIO	brawing	IISL	Horizontal Long Slotted Holes	OPP	Opposite	31111	3,
	Testing and Materials	/E\	Existing	ID	Inside Diameter	OSB	Oriented Strand Board	т	Тор
	All Weather Wood	(E) EA	Each	IN OR "	Inch	030	Oriented Strains Board	TEMP	Temporary
	7 III Weddier Wood	EC	Epoxy Coat	INCL	Include	Р	Plate	T&B	Top & Bottom
	Building	EF EF	Each Face			PAR	Parallel	T&G	Tongue & Groove
	Block			INSUL	Insulation	PC	Pre-Cast	THK or t	Thick
	Blocking	EJ	Expansion Joint	INT	Interior	PERIM	Perimeter	THRU	Through
	Beam	ELEC	Electrical	INTMD	Intermediate.	PLF	Pounds Per Linear Feet	TO	Top of
	Bottom	ELEV	Elevation					TOB	Top of Beam
	Bottom of	ENG	Engineer	JST	Joist	PLYWD	Plywood	TOC	Top Of Concrete
		EQ	Equal, Earthquake	JT	Joint	PMJ	Pre-molded Joint		
	Bearing	EQUIP	Equipment			PSF	Pounds Per Square Foot	TOS	Top of Steel
	Basement	EXP	Expansion	K	Long, Length, Angle	PSI	Pounds Per Square Inch	TOW	Top of Wall
	Between	EXST	Existing	KSI	Development Length	PT	Point	TRANS	Transverse
	0, , 0, ,;	EXT	Exterior			PT	Pressure - Treated	TS	Tube Steel
	Channel Section	EW	Each Way	L	Long, Length, Angle			TYP	Typical
	Cast-in-Place			LKD	Development Length	QTY	Quantity		
	Control Joint	FB	Flat Bar	LLH	Long Leg Horizontal			UBC	Uniform Building Code
	Ceiling	FD	Floor Drain	LLV	Long Leg Vertical	R	Radius, Reaction	UNO	Unless Noted Otherwise
	Clear	FDN	Foundation	LOC	Location	REF	Reference, Refer		
	Concrete Masonry Unit	FF	Finished Floor	LONG	Longitudinal	REINF	Reinforced	VER	Verify
	Column	FIN	Finish		•	REQ'D	Required	VERT	Vertical
Г	Construction Joint	FLR	Floor	MATL	Material	REV	Revision	VEST	Vestibule
	Concrete	FO	Face of	MAX	Maximum	RMV	Remove	VSL	Vertical Slotted Hole
	Connection	FOBM	Face of Beam	MC	Moment Connection/Misc. Channel	RM	Room		
₹	Construction	FOC	Face of Concrete	MECH	Mechanical	RO	Rough Opening	WH	Weep Hole
	Continuous	FOS	Face of Steel	MEMB	Membrane			WHS	Welded Headed Stud
	Contractor	FRMG	Framing	MFR	Manufacturer	SCHED	Schedule	W/	With
	Coordinate	FS	Far Side	MID	Middle	SECT	Section	w /0	Without
	Complete Penetration	FT	Foot or Feet	MIN	Minimum	SH P	SHEAR PLATE	••/ 0	Wide flange, Wide, Width
	Control Joint, Contraction Joint	FTG	Footing	MISC	Miscellaneous	SHT	Sheet	WD	Wood
	Center	FV	Field Verify	MTD		SIM	Similar	WP	Work Point, Water Proofing
	Countersunk	• •		MIU	Mounted	SI	Snow Load Slone	WE	Work Foilit, Water Frooting





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REHABILITATION PROJECT . EDA PROJECT NO. 07-79-04967

LUTAK DOCK REHA

DWG

DES. SHEET NO.

OR. CH.
F.B. or 16 SHEES

DATE

NOS. 40-02-010

LIST OF DRAWINGS

G1 COVER SHEET
G2 GENERAL NOTES
G3 CONTRACTOR WORK AND STAGING AREAS
C1 EXISTING SITE PLAN
C2 SITE GRADING PLAN
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D3 DREDGE DISPOSAL SITE PLAN AND SECTIONS
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CP2 CATHODE PROTECTION NOTES AND DETAILS

GENERAL NOTES

GENERAL

THE CONTRACTOR SHALL VERIFY AND COORDINATE ALL DIMENSIONS AMONG THE DRAWINGS BEFORE STARTING ANY WORK OR FABRICATION. ANY DISCREPANCIES FOUND AMONG THE DRAWINGS, SITE CONDITIONS, SPECIFICATIONS, AND THESE NOTES SHALL BE REPORTED TO THE OWNER'S REPRESENTATIVE.

ALL CONSTRUCTION SHALL COMPLY WITH THE STATE OF ALASKA DEPARTMENT OF TRANSPORTATION AND PUBLIC FACILITIES STANDARD SPECIFICATIONS.

SAFETY - THE CONTRACTOR IS RESPONSIBLE FOR MEETING ALL O.S.H.A. SAFETY STANDARDS. THE CONTRACTOR IS IN CHARGE OF ALL SAFETY MATTERS ON AND AROUND THE JOB SITE.

DEMOLITION

CONTRACTOR SHALL SUBMIT A DEMOLITION PLAN PRIOR TO BEGINNING WORK. PLAN SHALL CLEARLY DESCRIBE MEANS AND METHODS OF DEMOLITION AND IDENTIFY TO WHERE THE MATERIALS ARE TO BE TRANSPORTED AND BY WHAT MEANS.

THE EXISTING TIMBER FENDERS TO BE DEMOLISHED SHALL BE DISPOSED OF AT A LANDFILL AUTHORIZED TO RECEIVED CREOSOTE TREATED MATERIAL OR SHALL BE SALVAGED FOR REUSE BY THE CONTRACTOR. ALL OF THE TIMBER AND TIMBER PILES OF THE FENDERING SYSTEM ARE BELIEVED TO BE CREOSOTE TREATED.

THE NEW H-PILES, SHEET PILES AND CAP BEAM MODIFICATIONS SHALL BE COMPLETE BEFORE REMOVAL OF THE UPPER SECTION OF THE (E) CLOSURE ARCS AND THE FILL BEHIND THEM.

METHOD OF REMOVAL OF THE FILL BEHIND THE DEMOLISHED PORTION OF THE CLOSURE ARCS SHALL BE SUCH THAT THE CONDITIONS OF THE US ARMY CORPS OF ENGINEERS PERMIT FOR THIS PROJECT ARE NOT VIOLATED.

STRUCTURE DESIGN DATA

STRUCTURE MODIFICATIONS HAVE BEEN DESIGNED FOR THE FOLLOWING OPERATIONAL LOADS ON THE COMPLETED STRUCTURES. ADEQUACY OF EXISTING STRUCTURES TO SUPPORT THESE LOADS HAS NOT BEEN VERIFIED.

LIVE LOADS:

RO-RO BRIDGE MODIFICATIONS: 130 KIP AXLE WEIGHT

CLOSURE ARC MODIFICATIONS: 1000PSF UNIFORM LOAD OR

CRANE TRACK PRESSURE OF 3500 PSF OVER AREA 4'X23'

FENDER SYSTEM: VESSEL IMPACT ENERGY OF 230,000 FT - LBS @ 0 DEGREES

SPECIAL INSPECTION

THE FOLLOWING WORK SHALL BE INSPECTED BY A SPECIAL INSPECTOR:

- 1. CONCRETE. DURING THE TAKING OF TEST SPECIMENS AND PLACING OF REINFORCED CONCRETE.
- 2. BOLTS INSTALLED IN CONCRETE. PRIOR TO AND DURING THE PLACEMENT OF CONCRETE AROUND BOLTS.
- 3. REINFORCING STEEL. DURING PLACING OF REINFORCING STEEL. THE SPECIAL INSPECTOR NEED NOT BE PRESENT CONTINUOUSLY DURING PLACING OF REINFORCING STEEL PROVIDED THE SPECIAL INSPECTOR HAS INSPECTED FOR CONFORMANCE TO THE APPROVED PLANS PRIOR TO THE CLOSING OF FORMS OR THE DELIVERY OF CONCRETE TO THE JOBSITE.
- 4. STRUCTURAL WELDING. DURING THE WELDING OF ANY MEMBER, EXCEPT FOR WELDING DONE IN AN AISC APPROVED FABRICATOR'S SHOP. THE SPECIAL INSPECTOR NEED NOT BE CONTINUOUSLY PRESENT DURING WELDING OF SINGLE-PASS FILLET WELDS NOT EXCEEDING 5/16 INCH (7.9 MM) IN SIZE, PROVIDED THE MATERIALS, QUALIFICATIONS OF WELDING PROCEDURES AND WELDERS ARE VERIFIED PRIOR TO THE START OF WORK; PERIODIC INSPECTIONS ARE MADE OF WORK IN PROGRESS; AND A VISUAL INSPECTION OF ALL WELDS IS MADE PRIOR TO COMPLETION OR PRIOR TO SHIPMENT OF SHOP WELDING:
- 5. PILES. DURING DRIVING OF PILES

FOUNDATION NOTES

ALLOWABLE SOIL BEARING PRESSURE: 3000 PSF

CAP SUPPORT H-PILE CAPACITY REQ'D: 400 KIPS

REFER TO SOILS REPORT FOR PILE INSTALLATION, SUB-GRADE PREPARATION, AND LATERAL EARTH PRESSURES.

STRUCTURAL CONCRETE NOTES

ALL CONCRETE SHALL HAVE A WATER REDUCING ADMIXTURE MEETING ASTM C 494, TYPE A (TYPE E FOR COLD WEATHER CONCRETING) AND NOT MORE THAN 0.1 PERCENT CHLORIDE IONS. MAXIMUM WATER/CEMENT RATIO SHALL BE 0.40. MAXIMUM SLUMP BEFORE ADDING THE RANGE WATER REDUCING ADMIXTURE SHALL BE THREE INCHES. MAXIMUM AGGREGATE SIZE SHALL BE $\frac{3}{4}$. COLD WEATHER CONCRETING SHALL CONFORM TO ACI SPECIFICATION 306.1 AND ACI 306R. CALCIUM CHLORIDE SHALL NOT BE USED. MINIMUM CEMENT CONTENT SHALL BE 5-½ SACKS.

CAST-IN-PLACE CONCRETE: f'c = 4,000 PSI.

AIR ENTRAINING ADMIXTURE: ASTM C260

AGGREGATE: ASTM C33

EPOXY ADHESIVE: ASTM C881

REINFORCING BARS: ASTM A 615, GRADE 60.

WATER: ASTM C94, SECTION 4.1.3

CEMENT: ASTM C150

THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT FOR CAST-IN-PLACE CONCRETE:

A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH

TO EARTH 3 INCHES

B. ALL OTHER CONCRETE 2 INCHES

ALL WELDED WIRE FABRIC (WWF): ASTM A 185 OR ASTM A 497. CONTRACTOR SHALL TAKE SPECIAL CARE TO MAKE SURE WWF IN SLABS-ON-GRADE IS SUPPORTED IN ITS PROPER LOCATION.

WHERE REQUIRED, DOWELS SHALL MATCH SIZE AND QUANTITY OF MAIN REINFORCING.

NO WELDING OF REBAR IS ALLOWED WITHOUT ENGINEER'S APPROVAL.

DRILLED IN ADHESIVE ANCHORING

DRILLED IN ADHESIVE ANCHORING FOR BOTH ANCHOR BOLTS AND REBAR SHALL BE DONE WITH HILTI HVA 2421 EPOXY ADHESIVE OR AN APPROVED EQUAL. UNLESS OTHERWISE NOTED MINIMUM EMBEDMENT INTO CONCRETE SHALL BE 8 DIAMETERS. ALL INSTALLATION SHALL CONFORM TO THE MANUFACTURER'S INSTRUCTIONS.

STRUCTURAL STEEL NOTES

STRUCTURAL STEEL: ASTM A 36, UNLESS OTHERWISE NOTED.

STRUCTURAL STEEL TUBES (HSS): ASTM A 500, GRADE B.
STRUCTURAL STEEL PIPES: ASTM A 53, GRADE B.

BOLTS: ASTM A 325 EXCEPT WHERE NOTED A307

HARDENED WASHERS: ASTM F 436

NUTS: AS RECOMMENDED BY ASTM IN THE BOLT SPECIFICATION

 WELDED HEADED STUDS:
 ASTM A 108

 THREADED RODS:
 ASTM A 36 OR A307

 NON-SHRINK GROUT:
 ASTM C 1107

GALVANIZING: ALL STEEL FABRICATION AND PILING (EXCEPT SHEET PILES) SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A 123, UNLESS OTHERWISE NOTED.

STEEL SHEET PILES

WELDING ELECTRODES :

SHEET PILES SHALL BE "Z" - SHAPED, SHALL HAVE A 50 KSI MINIMUM YIELD STRENGTH, AND SHALL, AS A MINIMUM, HAVE THE FOLLOWING SECTION PROPERTIES PER LINEAL FOOT OF WALL:

E70. LOW-HYDROGEN

AREA _IN ² _	SECTION MODULUS	MOMENT OF INERTIA	WEIGHT PSF
11.2	46.8	281	.38

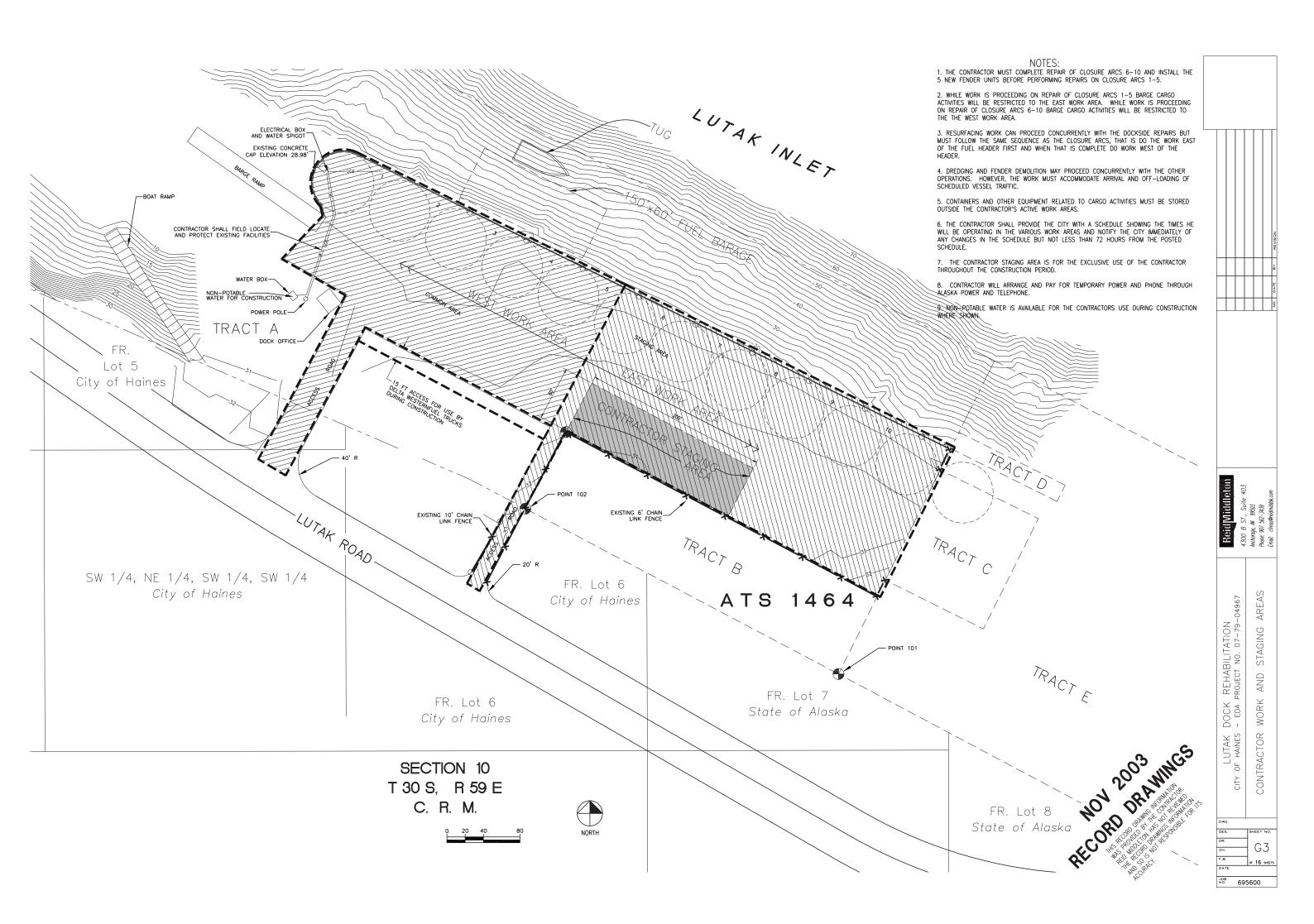
LUTAK DOCK REHABILITATION

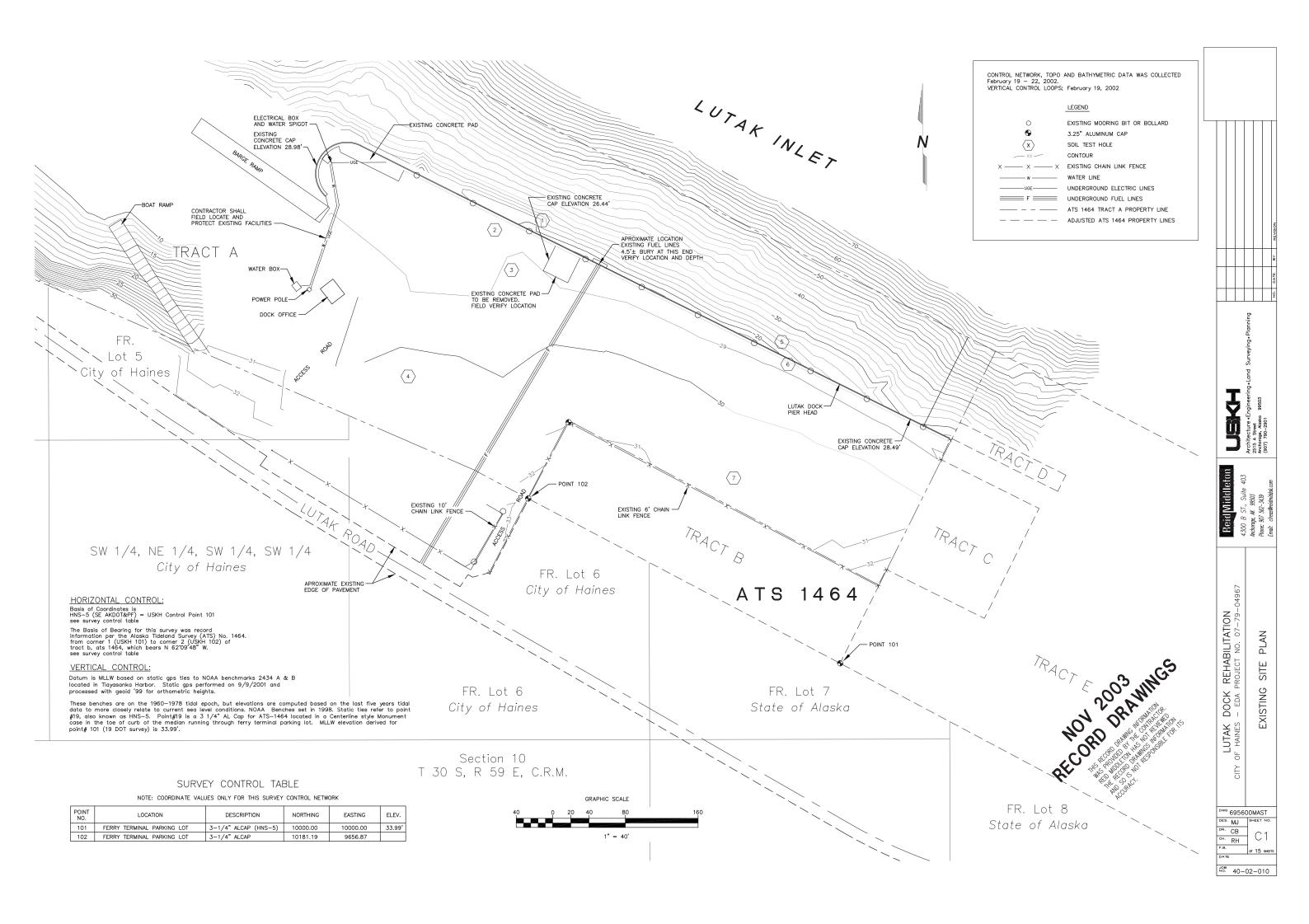
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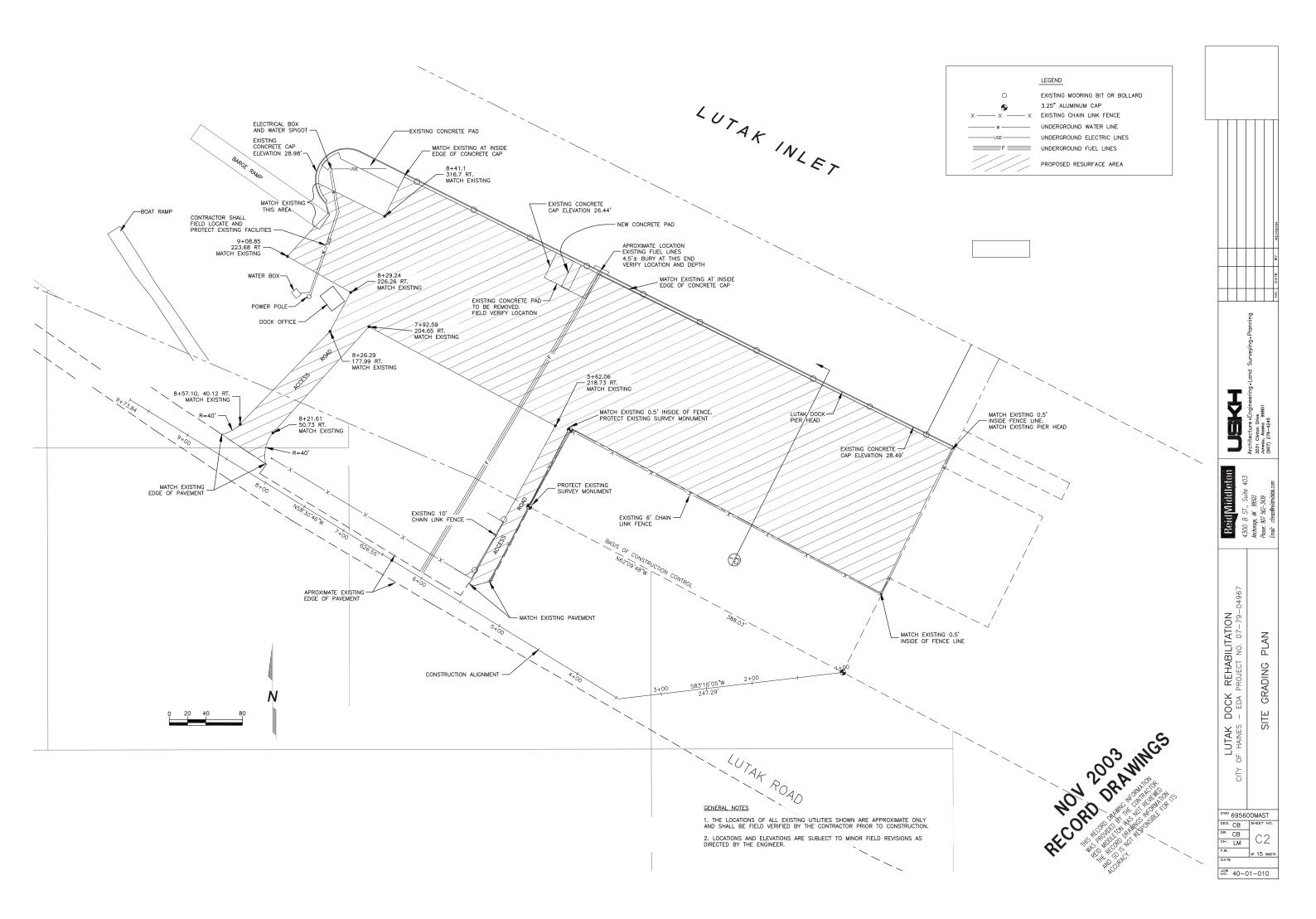
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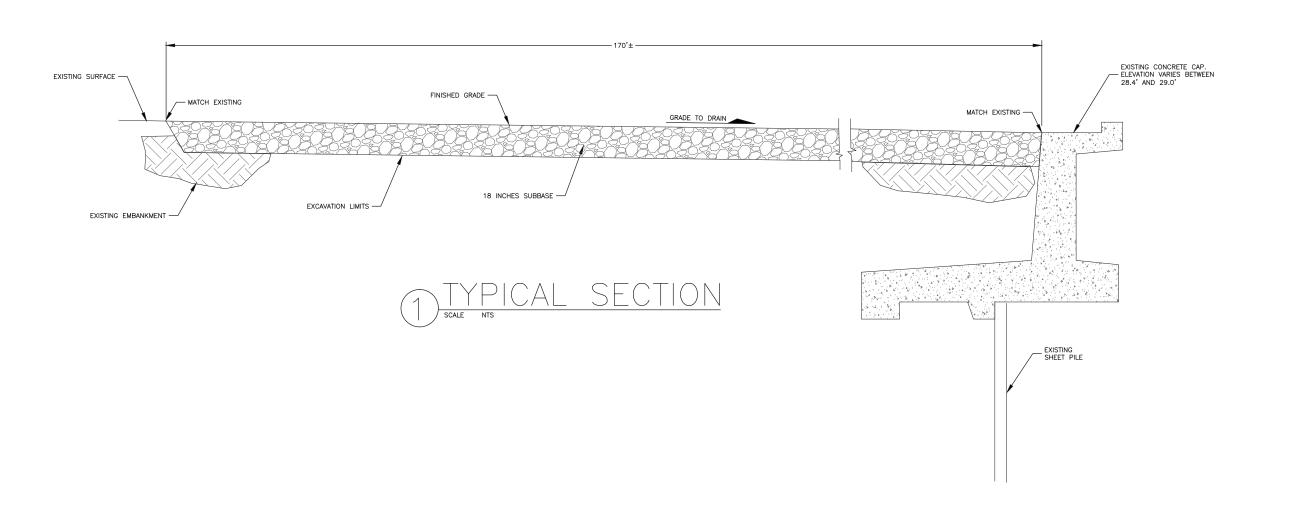
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DES. SHEET NO.
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CITY OF HAINES - EDA PROJECT NO. 07-79-04967 CIVIL DETAILS

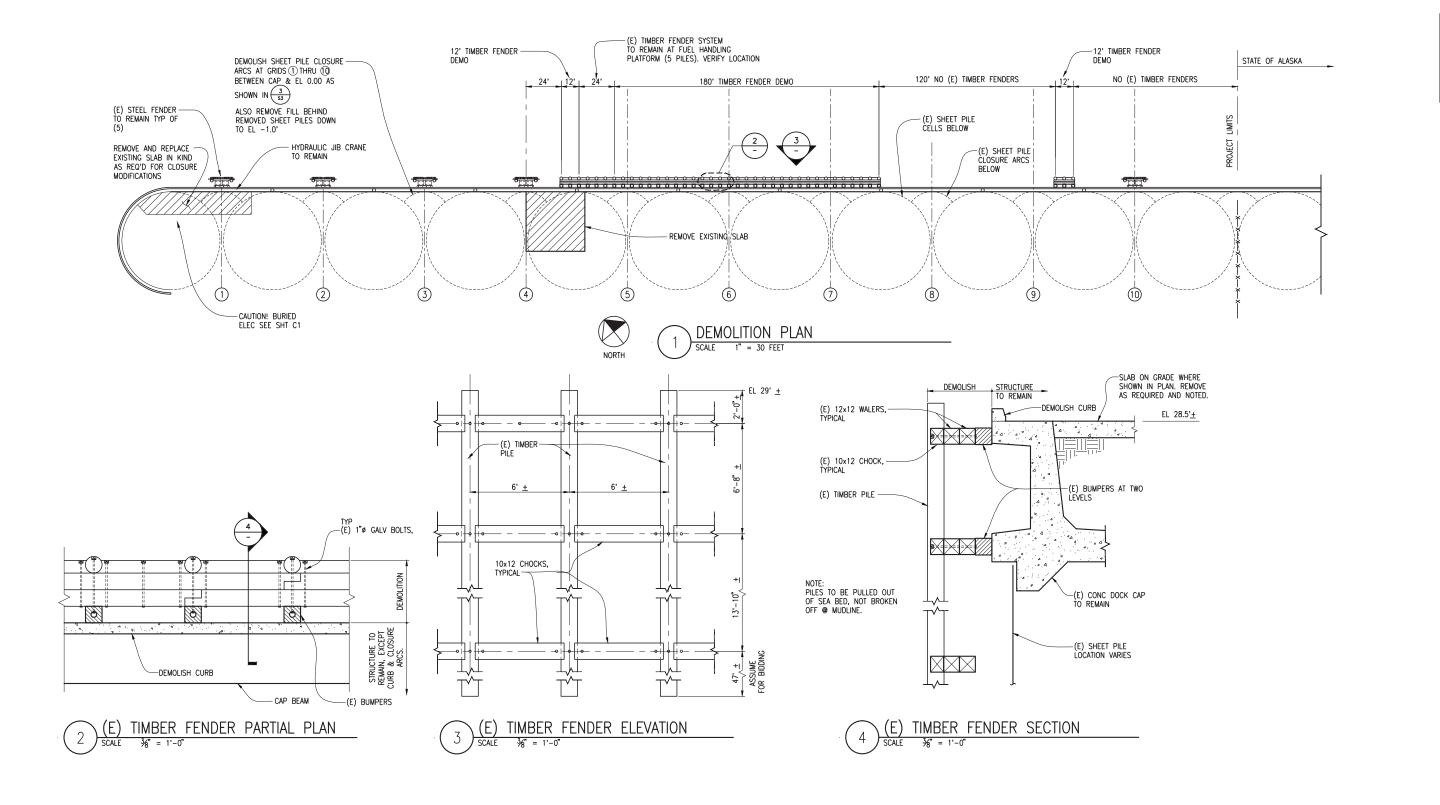
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OF 15 SHEETS

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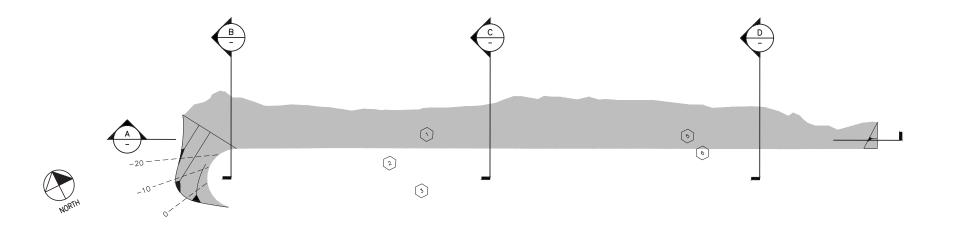


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10 B ST., Suite 403 orage, AV 99503 e: 907 562-3439 LUTAK DOCK REHABILITATION PROJECT CITY OF HAINES - EDA PROJECT NO. 07-79-04967 DEMOLITION PLAN

D 1

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(A)

LEGEND



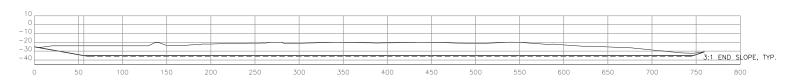
DESIGN DREDGE PRISM (-35.0')



DREDGE PLAN SCALE: 1"=50'

ESTIMATED BOTTOM CONTOURS FROM PREVIOUS SURVEY AT BARGE RO/RO RAMP

 \bigcirc





HORIZONTAL CONTROL:

BASIS OF COORDINATES IS HNS-5 (SE AKDOT&PF) = USKH CONTROL PT 101 SEE SURVEY CONTROL TABLE

THE BASIS OF BEARING FOR THIS SURVEY WAS RECORD INFORMATION PER THE ALASKA TIDELAND SURVEY (ATS) No. 1464. FROM CORNER 1 (USKH 101) TO CORNER 2 (USKH 102) OF TRACT B, ATS 1464, WHICH BEARS N 62'09'48" W. SEE SURVEY CONTROL TABLE

VERTICAL CONTROL:

SOURCE OF ELEVATION - SE AKDOT&PF BENCH MARK (HNS-5)

ELEV. = 33.99 ft. (MLLW)

DESC. = ALUMINUM CAP IN A MONUMENT CASE ALONG THE OF A CURB OF THE MEDIAN RUNNING THROUGH FERRY TERMINAL PARKING LOT.

SURVEY CONTROL TABLE

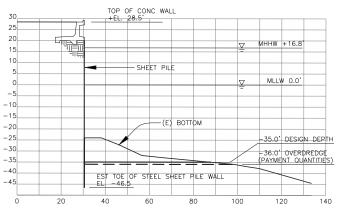
NOTE: COORDINATE VALUES ONLY FOR THIS SURVEY CONTROL NETWORK

POINT NO.	LOCATION	DESCRIPTION	NORTHING	EASTING	ELEV.
101	FERRY TERMINAL PARKING LOT	3-1/4" ALCAP (HNS-5)	10000.00	10000.00	33.99
102	FERRY TERMINAL PARKING LOT	3-1/4" ALCAP	10181.19	9656.87	

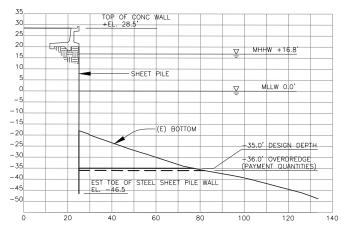
CONTROL NETWORK, TOPO AND BATHYMETRIC DATA WAS COLLECTED February 19 - 22, 2002. VERTICAL CONTROL LOOPS; February 19, 2002 SURFACE = 6956A Existing ground LEGEND BOLLARD TEST HOLE SHANNON & WILSON FEB 2002 CONTOUR -132.5 -CHAIN LINK FENCE ATS PROPERTY LINE ADJUSTED PROPERTY LINE RESURFACE AREA

TIDAL DATUM

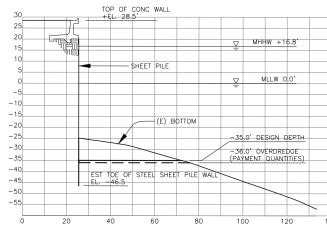
HAINES, CHILKOOT INLET, LYNN CANAL LAT. 59°13.8' LONG. 135°26.0' ESTIMATED EXTREME HIGH WATER (EHW) +22.5'
MEAN HIGHER HIGH WATER (MHHW) +16.8'
MEAN HIGH WATER (MHW) +15.8' MEAN TIDE LEVEL (MTL) +6.7' MEAN LOW WATER (MLW) +1.6' MEAN LOWER LOW WATER (MLLW) 0.0' ESTIMATED EXTREME LOW WATER (ELW) -6.0'



В



SECTION SCALE 1" =





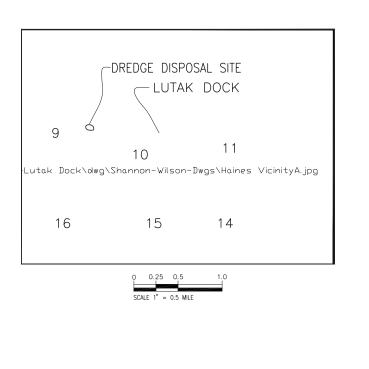
ESTIMATED	DREDGE QUANT	ITIES
DESIGN EL35.0'	<u>VOLUME (CY)</u> 9,620	<u>AREA (SF)</u> 39,545
1 FT OVERDREDGE	1,380	3,765
TOTAL (EL36.0')	11,000	43,310



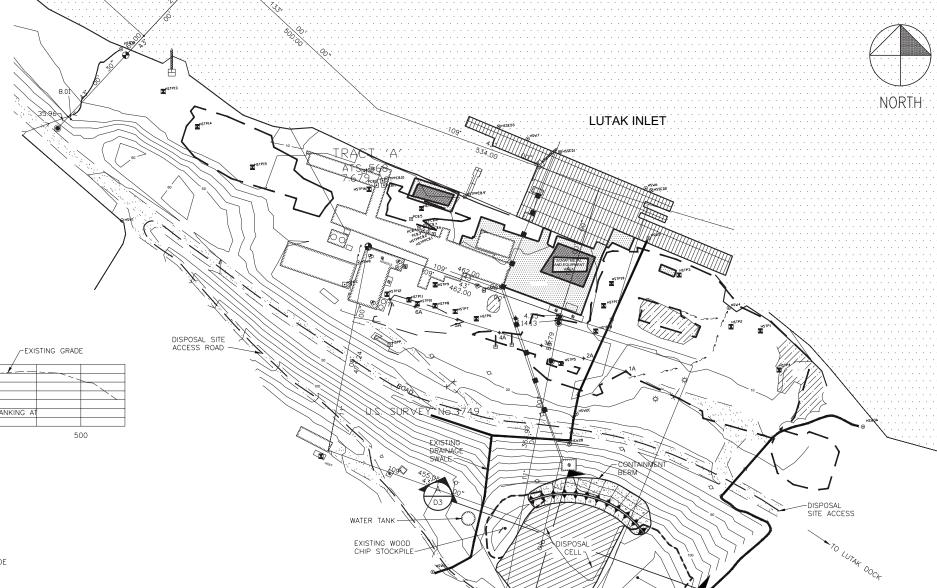
	· 5	LUTAK DOCK REHABILITATION PROJECT CITY OF HAINES - EDA PROJECT NO. 07-79-04967	DREDGING PLAN AND SECTIONS
1		DES.	SHEET NO.

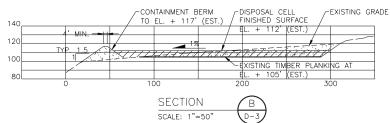
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D2 JOB 40-02-010



100





-DISPOSAL CELL FINISHED SURFACE EL. # 112' (EST.)

300

SECTION SCALE: 1"=50"

NOTES:

- 1. DISPOSAL SITE WAS HISTORICALLY USED FOR WOOD CHIP STORAGE AND IS UNDERLAID WITH TIMBER PLANKING. OWNER REPORTS APPROXIMATELY 3 FEET THICK LAYER OF WOOD CHIPS (TYPICAL) ACROSS DISPOSAL SITE. SITE IS BOUNDED BY WOOD CHIP STOCKPILE TO WEST AND HOG FUEL STOCKPILE OVER ROCK OUTCROP TO EAST.
- 2. IMPROVE ACCESS ROAD AS REQUIRED FOR SITE ACCESS AND DISPOSAL OPERATIONS.
- 3. CONSTRUCT CONTAINMENT BERM ALONG NORTHERN (DOWN SLOPE) LIMIT OF DISPOSAL CELL, USING WOOD CHIPS FROM THE EXISTING STOCK PILE AND TO THE EXTENT NECESSARY, WOOD CHIPS FROM THE DISPOSAL CELL AREA OVER THE EXISTING TIMBER PLANKING. POSITION CONTAINMENT BERM WITHIN LIMITS SHOWN TO FACILITATE CONSTRUCTION.
- 4. SPREAD DREDGE MATERIAL EVENLY THROUGHOUT DISPOSAL CELL TO CREATE A CONSISTENT 1% DOWN SLOPE GRADE.
- MAINTAIN CONTAINMENT BERM MINIMUM 5 FEET ABOVE FINISHED GRADE OF DISPOSAL MATERIAL CELL. BORROW FROM WOOD CHIP STOCKPILE AS NEEDED TO INCREASE CONTAINMENT DURING CONSTRUCTION.
- INTENT OF CELL CONTAINMENT IS TO PREVENT FREE DRAINAGE OF DISPOSAL MATERIAL INTO EXISTING DRAINAGE SWALES. MODIFY AND MAINTAIN CONTAINMENT THROUGHOUT CONSTRUCTION AS REQUIRED.

-EXISTING HOG FUEL STOCKPILE OVER ROCK OUTCROP

-EXISTING DRAINAGE SWALE

CITY OF HAINES - EDA PROJECT NO. 07-79-04967 LUTAK DOCK REHABILITATION PROJECT AND PLAN SITE DREDGE DISPOSAL

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SECTIONS

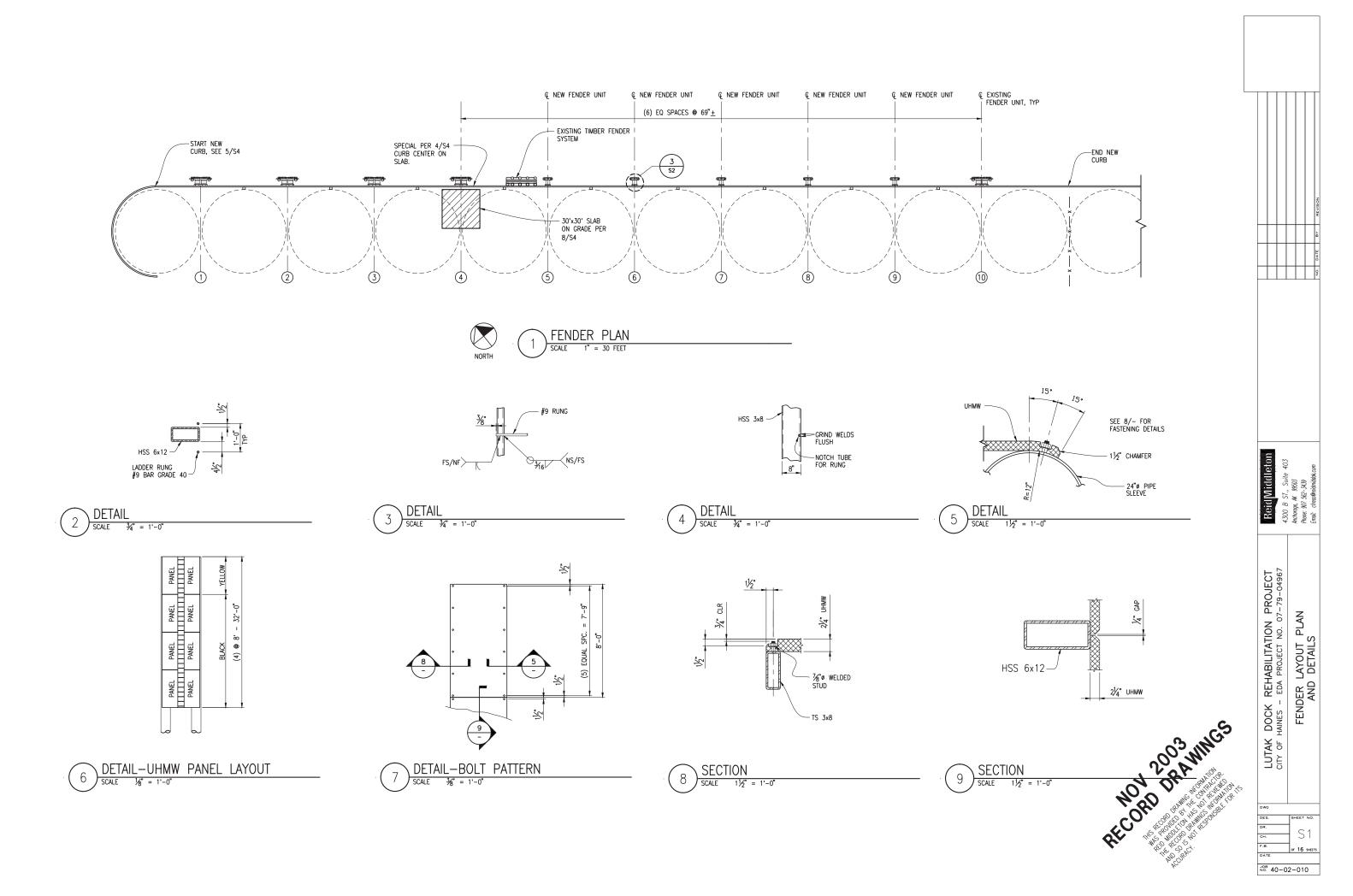
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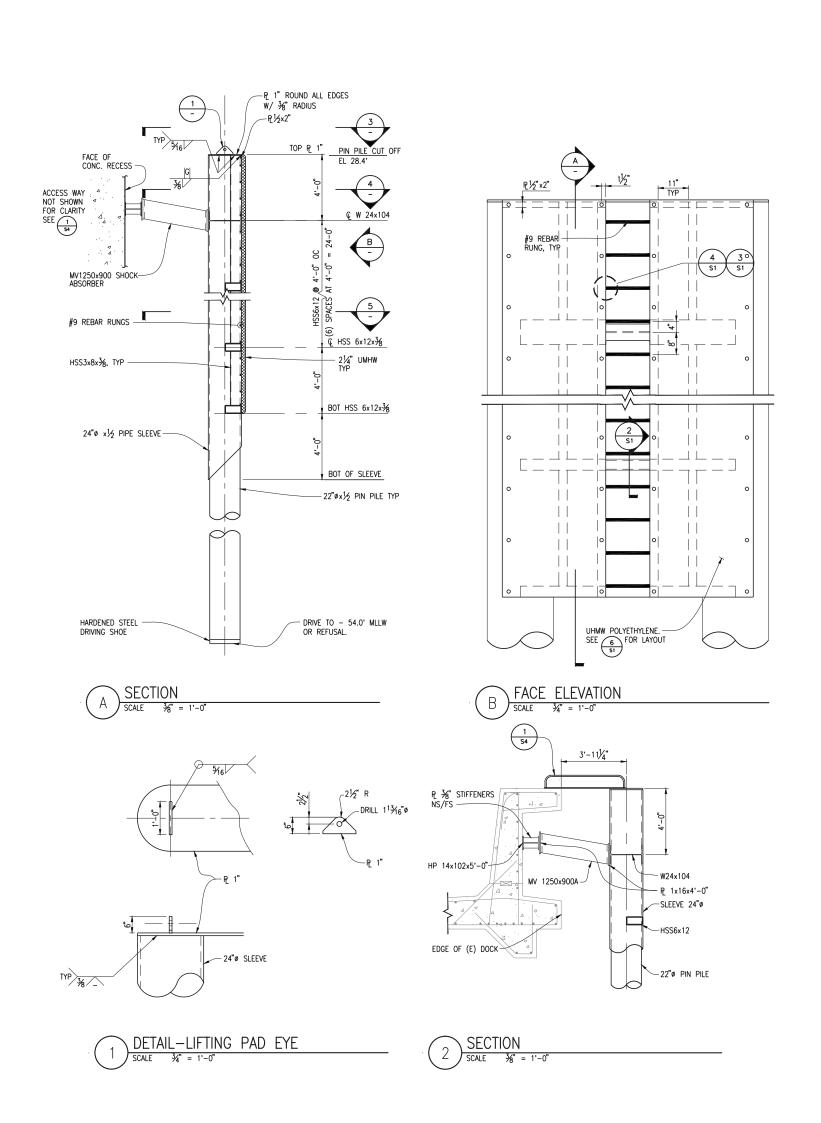
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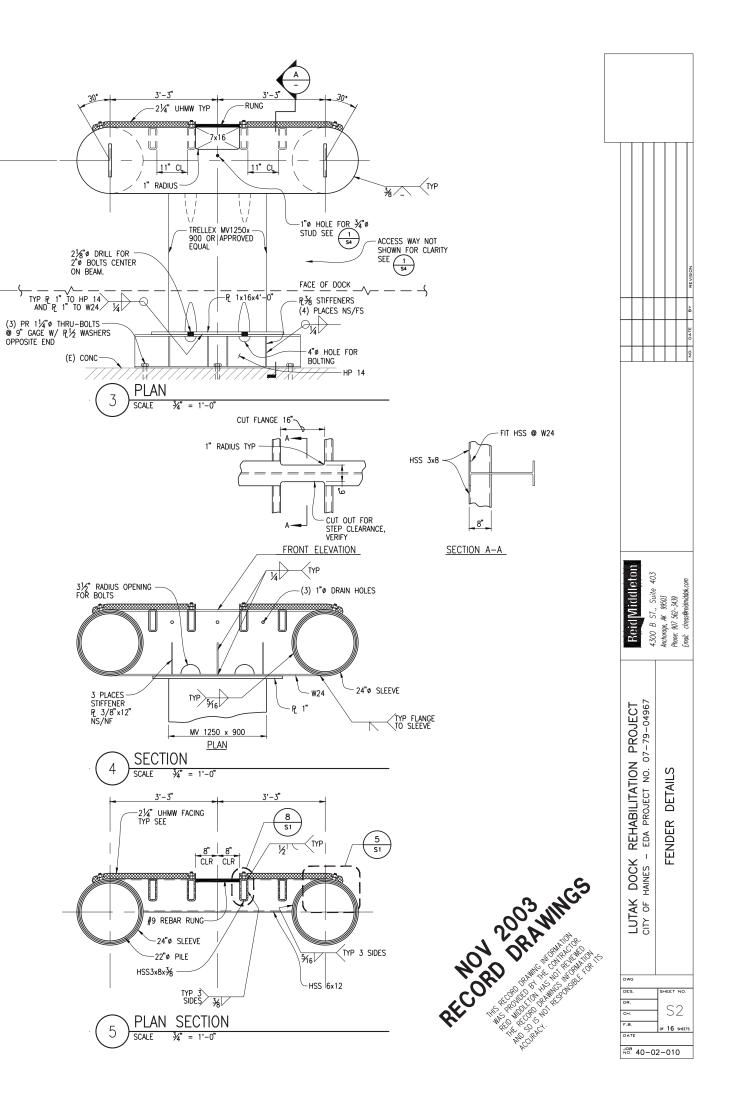
EXISTING DRAINAGE SWALES (INDICATING DIRECTION OF FLOW)

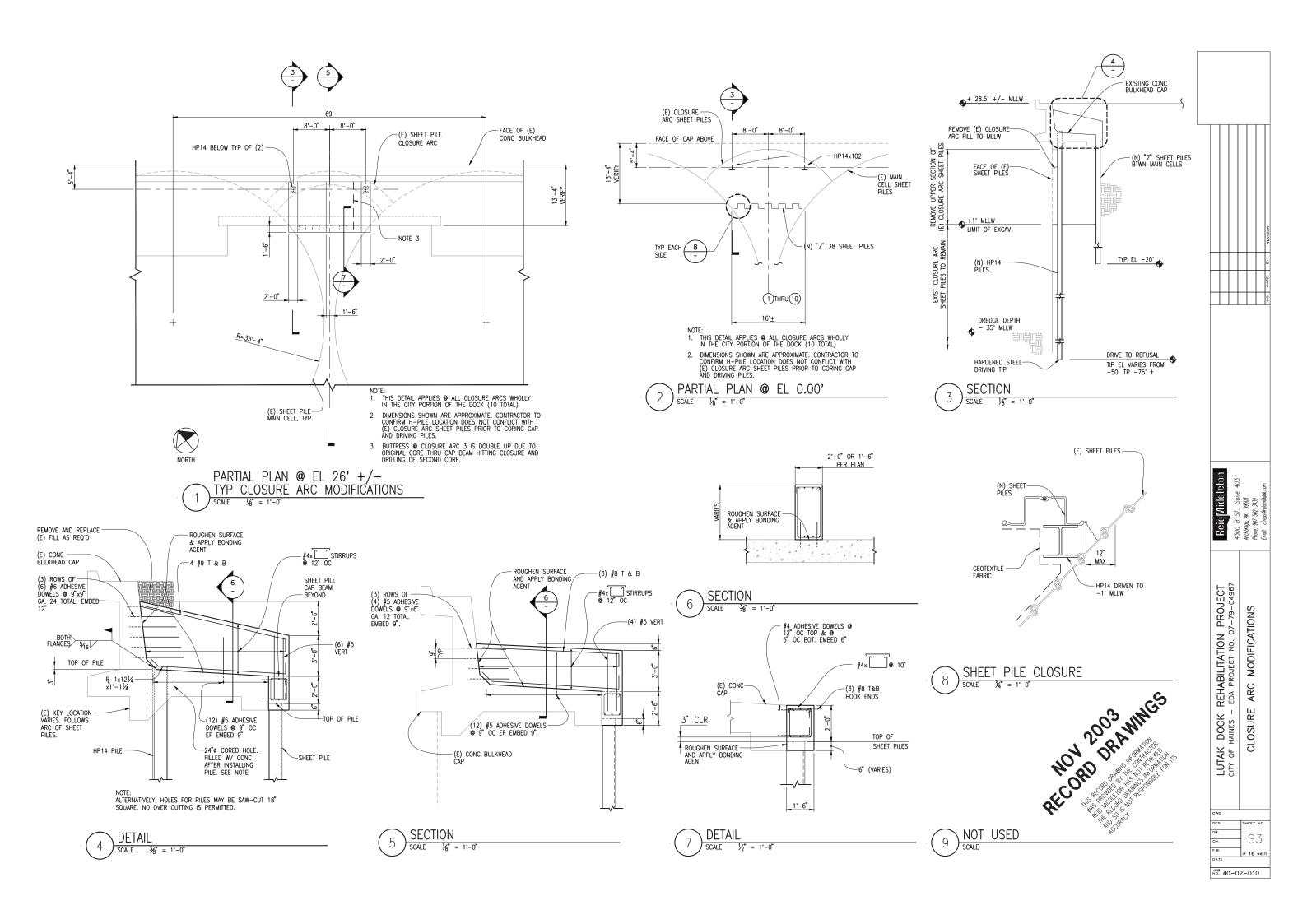
wood CHIPS

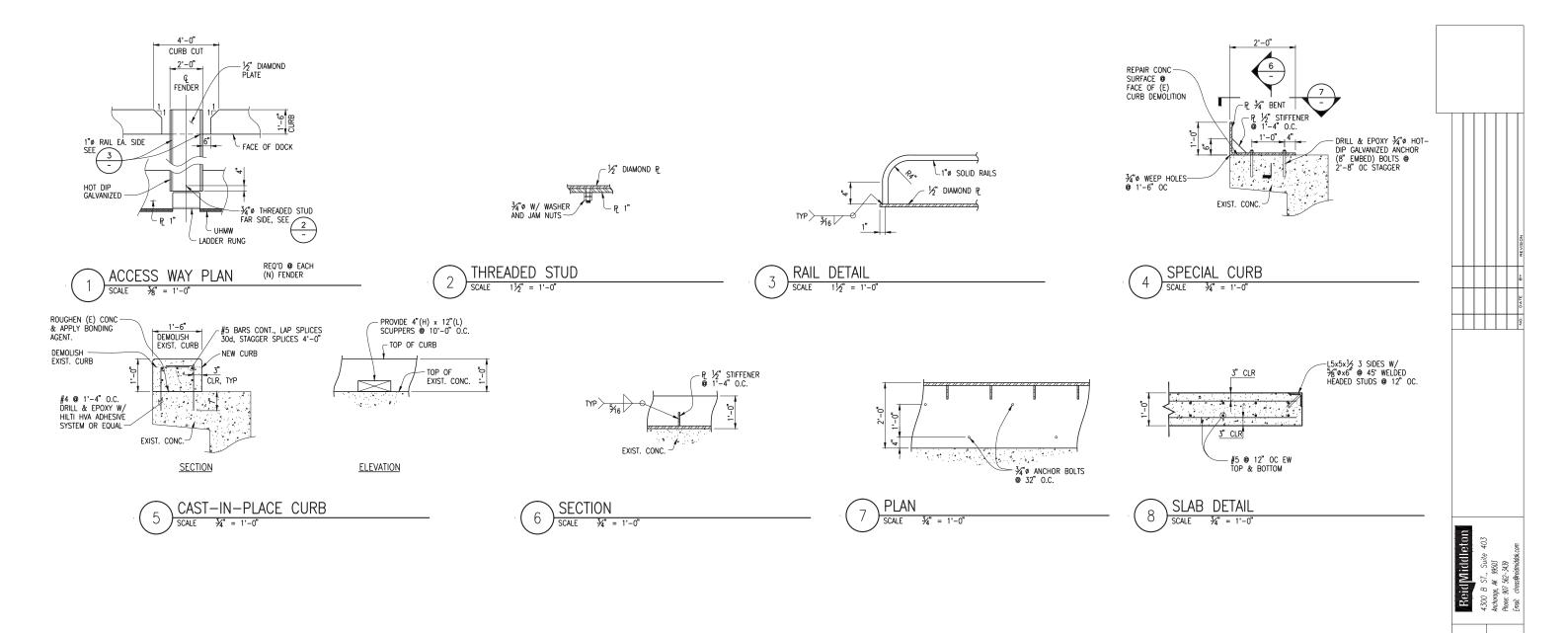
DREDGE MATERIAL DISPOSAL











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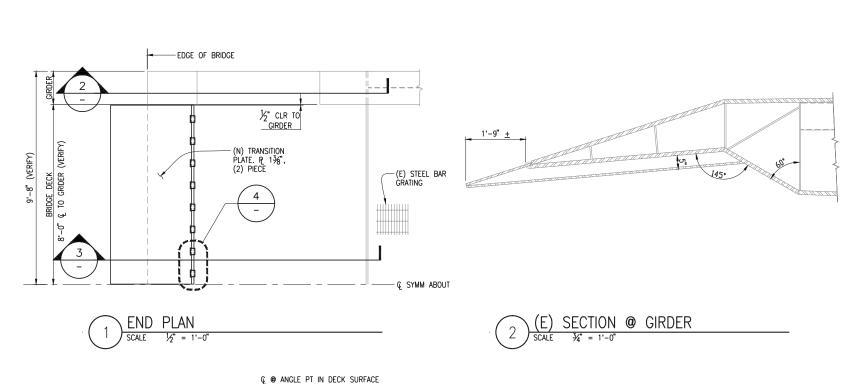
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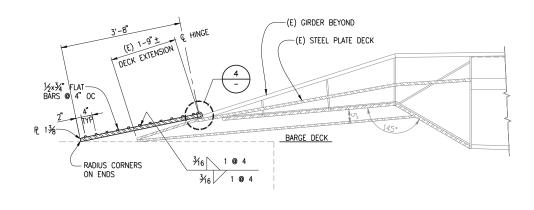
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LUTAK DOCK REHABILITATION PROJECT CITY OF HAINES - EDA PROJECT NO. 07-79-04967

MISCELLANEOUS DETAILS









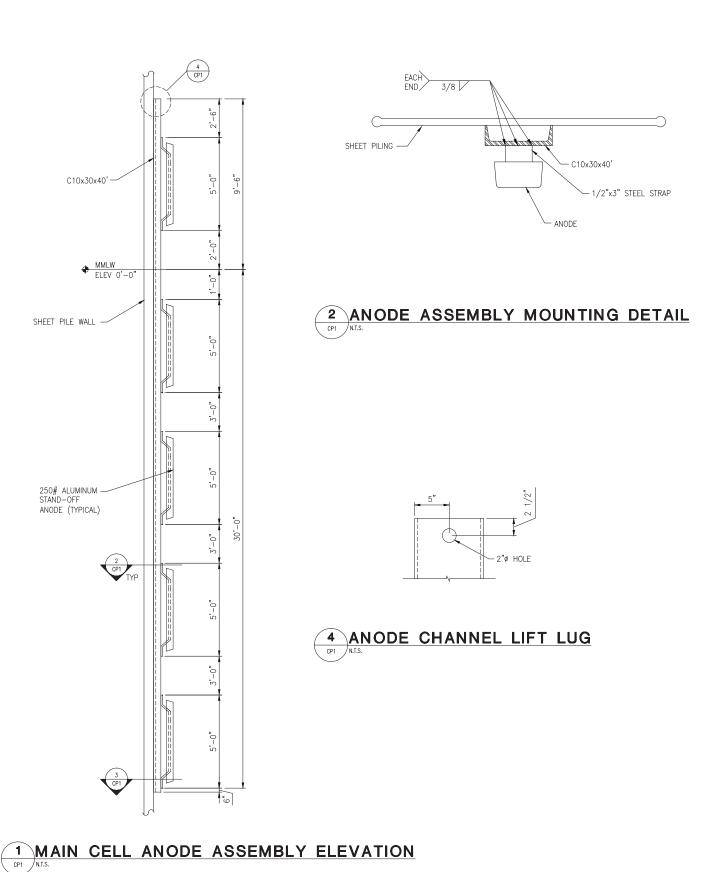
LUTAK DOCK REHABILITATION PROJECT CITY OF HAINES - EDA PROJECT NO. 07-79-04967 RO-RO TRANSFER BRIDGE

S5 JOB 40-02-010

BAR TO PLATE (N) PLATE —(E) BRIDGE DECK SECTION A-A 13∕8″ø BAR P3%x3½" WIDE BENT U STRAP — Ç DECK STRAP PLAN HINGE DETAIL

SCALE 1½" = 1'-0"

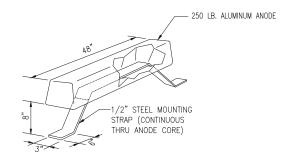
NOTE: SEE SHEET C1 FOR TRANSFER BRIDGE (BARGE RAMP) LOCATION



C10x30x40' W/ 3/4"Ø HOLES @ 24" O.C. LENGTH OF CHANNEL. 5/16 12 SHEET PILING -SHEET PILING -(2) %"ø WELDED THREADED STUDS @ 6" GA. 5/16 12 PL ½x12x1'-0", TYP - NO PAINT @ INTERFACE ANODE MID HEIGHT TOP & BOT

3 CHANNEL MOUNTING DETAIL

OP1 NT.S.



5 STAND-OFF ANODE DETAIL

OP1 N.T.S.



ACCEPTANT 6 IN E R 8 S 800 F Street 800 F St

CATHODIC PROTECTION DETAILS

LUTAK DOCK REHABILITATION PROJECT CITY OF HAINES - EDA PROJECT NO. 07-79-04967

DES. CDS SHEET NO.

DR. LAI

CH. TB

F.B.

OF SHEET JOB 40-02-010 1.SPACE ANODE ASSEMBLIES EQUAL DISTANCE BETWEEN EXISTING CLOSURE ARCS ON MAIN CELLS.

2.START ASSEMBLY ORIENTATION AS CLOSE AS POSSIBLE TO EXISTING ARC

FXISTING -

MAIN CELL

(TYPICAL)

1 ANODE ASSEMBLY ORIENTATION

CATHODIC PROTECTION NOTES

GENERAL

- A. The Contractor shall provide all labor, equipment and materials required for a complete galvanic anode cathodic protection system. The cathodic protection system shall include the appropriate anodes, channels and all necessary items of equipment and materials for a completed system.
- B. Corrosion control materials shall be submitted to the Engineer for approval. A complete list of equipment and materials, including name and manufacturer, catalog number, size, finish and other pertinent data shall be provided. The following items shall be included in the
- Aluminum anodes
 Epoxy Coating
- 3. Channels (Mill test certificates)
- 4. Welders qualifications & weld procedures

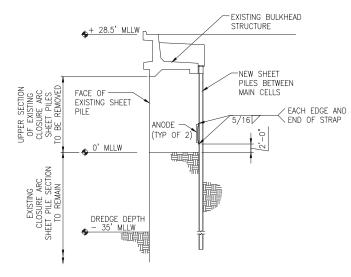
PRODUCTS

- A. All materials shall conform to the requirements set forth herein or as shown. All materials must be new, free from defects, and shall be of the best commercial quality for the purpose specified. All necessary items and accessories not shown or specified herein, but which are required to fully carry out the specified intent of the work, shall be furnished without
- B. Aluminum stand-off anode shall be 250 lb. weight and shall have a 1/2"x3" steel mounting strap continuous through the core. Anodes shall conform to the following chemical composition. The 250 lb. weight is the weight of the anode material and excludes the weight of the strap.

Element Indium Zinc Silicon 3.0 to 5.0 0.08 max. 0.09 max. Mercury 0.001 max. 0.001 max. Others(each) Aluminum 0.097% max

"The chemical composition noted above is the chemical composition of the Purity Casting Alloys anodes. The standard chemical composition of the Galvaium III offshore aluminum anodes is also acceptable. Other anode composition must be approved by the Owner's Representative.'

- C. Coating shall be Sherwin-Williams Dura-Plate 235 or Engineer approved equal. Color shall be gray.
- D. Channels shall be C10x30x40', ASTM A36 steel.
- E. Welds shall use 60 KSI minimum rod. Welders and procedures shall be qualified per A.W.S.



NOTES:

- 1.INSTALL TWO(2) NEW ANODES AT EACH CLOSURE ARC LOCATION, SPACE ANODES FOUAL DISTANCE (HORIZONTALLY) BETWEEN MAIN CELLS.
- 2.INSTALL EACH ANODE APPROXIMATELY FIVE(5) FEET IN FROM THE MAIN CELL WALLS.

2 CLOSURE ARC ANODE INSTALLATION DETAIL

EXECUTION

- A. Anodes shall be installed as shown. Anodes shall be installed on channel assemblies on the main cells and directly attached to the new sheet pile closure arcs. Anodes, damaged as a result of improper handling, shall be replaced.
- B. Remove all marine growth and clean all surfaces that the anodes and anode channel are being installed to.
- C. Each end of the anode straps shall be welded to the anode channel or the new closure arc sheet pile. A continuous weld is required on each side and the end of
- D. The anode channels shall be welded to the main cells at the top, bottom and mid-height of the channels. Both sides of the channel shall be welded at these
- E. Main cell anodes assemblies and individual anodes being installed on the new sheet piling shall be installed at equal distances between anodes/assemblies.
- F. Coat the mounting straps, channels, and welds of the anode assembles and individual closure arc anodes, after fabrication, with one coat (4 to 8 mills DFT) of the coating per the manufacturer's recommendations. Ensure that no coating is applied to the anode surface. Touch—up of the coating at the welds to the sheet
- G. All welders shall be AWS (or equal) qualified for the welding and procedures performed by them. Provide certificates of their testing and qualification for all procedures, including wet welding procedures.
- H. Weld Inspection: Coordinate dry weld visual inspections with the Owner's Representative. Owner's Representative will visually inspect welds of anodes to channels, prior to coating the assemblies, and will visually inspect the dry welds of the anode assemblies to the sheet piles. Contractor shall provide a photographic record (video or still photography) of all wet welds for review by the Owner's

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PROJECT 7-79-04967 AND REHABILITATION EDA PROJECT NO. 0

DES. CDS ^{1.} ТВ

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